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2016 September 16

**Dr. Sukhi Muker** 1785 Bellevue Avenue West Vancouver, BC V7V 1A8

Via email: sukhi@drsukhi.com

Subject: 4701 Piccadilly Place South - Creek Realignment Hydrology and Hydraulic Design – 2016 Update for Revision 2 Drawing

#### **1** INTRODUCTION

Northwest Hydraulic Consultants Ltd. (NHC) is pleased to present this technical report which outlines the design for the realignment of an ephemeral<sup>1</sup> watercourse at 4701 Piccadilly Place South in the District of West Vancouver (DWV).

The property is bounded by Marine Drive to the north and Piccadilly Drive to the south. The DWV right of way, containing Clovelley Walk and a subsurface storm utility, bound the property on its west side and the residential property of 4699 Piccadilly bound the property to the east. An unnamed watercourse bisects the property from north to south (**Figure 1**).

It is our understanding that the existing house on the property is to be replaced with a new structure and the present alignment of the watercourse is undesirable for the new building footprint. The attached drawing presents the current design for development of the property.

Re-development of the property and subsequent realignment of the watercourse triggers a number of concerns with respect to the DWV;

- 1. DWV watercourse protection Bylaw 4364 Section 6 states that: "open watercourses and ditches shall remain above ground and may only be enclosed where no alternatives exist, as determined by the District".
- 2. DWV requires an Environmental Development Permit for any work within 15 m of a water course.

<sup>&</sup>lt;sup>1</sup> watercourse that only holds water during and immediately following rain events.



3. Realignment is expected to result in a no-net loss of riparian habitat (defined by DWV as area within 5 m of the top of bank for non-permanent watercourse).

Prior to the issuing of building permits for the property, DWV requires engineering drawings and a report of the channel realignment for authorization. This report and the enclosed drawings are to be submitted for acquisition of a building permit.

### 1.1 Site Visit

Prior to NHC being retained a site visit was conducted with Jas Muker (property developer), November 11<sup>th</sup>, 2014. At this time, initial information of the channel properties, drainage patterns, and realignment constraints were gathered.

A secondary visit was conducted on December 12<sup>th</sup>, 2014 by Joanna Glawdel and Justin Finn of NHC to collect additional topographic survey and details for channel alignment design.

#### **1.2** Watercourse Description

The watercourse crossing the property may have once been a natural small creek (Ven Huizen Consultants Ltd., 2014). However alterations to the creek upstream, downstream and on the property have limited the water supply and structured the watercourse to its current unnatural form which includes concrete and angular rock lining.

The watercourse originates at the northeast end of the property at the outlet of a corrugated metal pipe under Marine Drive (**Figure 1** and **Photo 1**). During the site visit in December the culvert was dry, despite intense precipitation the previous 4 days; as stated in **Table 1** for the Environment Canada West Vancouver gauge (1108824). Upstream property and road drainage appears to be directed to other systems such as the DWV storm system under Clovelley Walk.

Day	Daily Precipitation (mm)			
8	18.1			
9	46.3			
10	46.4			
11	13.2			
12	0.8			

 Table 1. West Vancouver Precipitation December 2014, (Environment Canada – Climate ID 1108824)

In addition, a small drainage channel, which was likely constructed by previous owners, runs west to east along the north boundary of the property (**Figure 1** and **Photo 2**). Regular flow to the drainage channels appears to have previously been provided by a piped and valve controlled connection from the DWV storm utility (now sealed) (**Photo 3**). The drainage channel currently only intercepts seepage and overflow from Marine Drive and directs it to the unnamed watercourse 4 m downstream of the northern property boundary (**Photo 4**). Similar to the unnamed watercourse, the drainage channel is expected to only convey flow during the most extreme events.



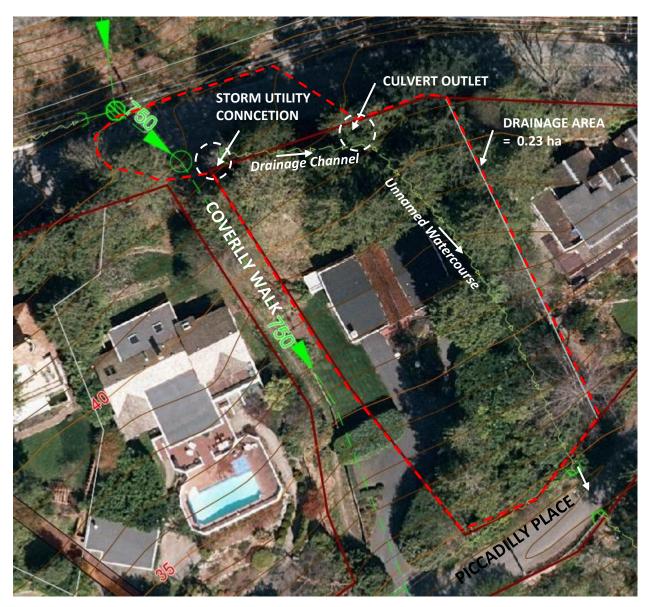


Figure 1. Project Overview and Drainage Area (drainage network and orthophoto from DWV GIS).

Downstream of the confluence with the drainage channel, the watercourse runs through a vegetated area with well-defined banks for 14.5 m (**Photo 5**). After this there is an 8.5 m long concrete lined channel under the deck of the existing house (**Photo 6**). The watercourse then outlets to an approximately 9 m long by 3 m wide and 0.5-1.0 m deep man-made pond where water level is controlled by a concrete weir at the pond's outlet (**Photo 7**). The pond outlets through a small 22 m long grass lined ditch to a rock outcrop (approximately 2.3 m from the properties south boundary), where flow spills over without a defined channel or path (**Photo 8**). At the base of the rock, surface flow is collected by the road drainage ditch and directed through a culvert under Piccadilly Place South. Flow then continues south in an approximately 0.6m wide by 0.3 m deep rock lined channel along 4704 Dogwood Lane (**Photo 9**) prior to discharging to Dogwood Creek and subsequently Burrard Inlet.



# 2 CHANNEL ALIGNMENT

The preferred alignment of the channel by the property owner is on the east side of the existing building which follows the natural drainage slope of the property. The proposed channel realignment is intended to not restrict the future development envelope of the east neighbouring property (4699 Piccadilly Place) such that the entire 5 m riparian area is located within the property of 4701 Piccadilly Place. Hence, the top of bank of the realigned channel is to be no closer than 5.0 m from the property boundary, to avoid placing Environmental Development Permit (EDP) requirements on future development of the neighbouring property<sup>2</sup>.

Additional topographic survey data was collected by NHC in December 2014 to identify additional channel alignment constraints such as exposed bedrock and mature trees and their expected root systems.

# **3** HYDROLOGY

The drainage area for the channel is conservatively estimated as 0.23 ha which was determined from field investigation and site survey and is shown on **Figure 1**.

Flow data is not available at the site. The Rational Formula has been used to estimate design peak flows, with runoff coefficients and time of concentration estimated from the *District of North Vancouver (DNV) Design Criteria Manual* (2006). The general form of the Rational Formula is as follows:

$$Q = RAIN \tag{1}$$

Where

R = runoff coefficient (C) x soil adjustment factor (SAF) A = drainage area in hectares (ha) I = rainfall intensity in mm/h N = 0.00278

For return period flows greater than 10-years, a runoff coefficient, C, of 0.6, was estimated for the drainage area which has an impervious area of approximately 46%; a C value of 0.55 was applied for flows less than 10-years. A soil adjustment factor of 1.0 was applied given the permeability of the soil and slope of the drainage area. The time of concentration, T<sub>c</sub>, was estimated to be 10 minutes as recommended by BC MoT (2007) for a residential area and is consistent with recommended time of concentrations in DNV (2006).

The Metro Vancouver (MV) rain gauge at the West Vancouver Firehall (VW 49) (**Figure 2**) was used to estimate precipitation at the site due to its proximity to the site. The station contained data for the

<sup>&</sup>lt;sup>2</sup> For a non-permanent watercourse, the DWV requires an EDP for work within 5 m of the top of bank. It is understood by NHC that in Dr. Sukhi Muker's discussions with the neighbouring property owners and DWV staff that the riparian area is to be located within the property of 4701 Piccadilly Place.



period of 1995 to 2012, thus providing 18 years of record. The IDF curve was used un-altered for the site and was provided by Metro Vancouver for use in this study.

Peak instantaneous discharge estimates are provided in **Table 2** below with respect to the return period (the rainfall intensity or "I" in Equation 1, are also provided for reference).

Return Period (Years)	10 min Rainfall Intensity (mm/hr)	Peak Instantaneous Discharge (L/s)
2	26.3	9.3
25	57.0	22
50	64.7	25
100	72.3	28

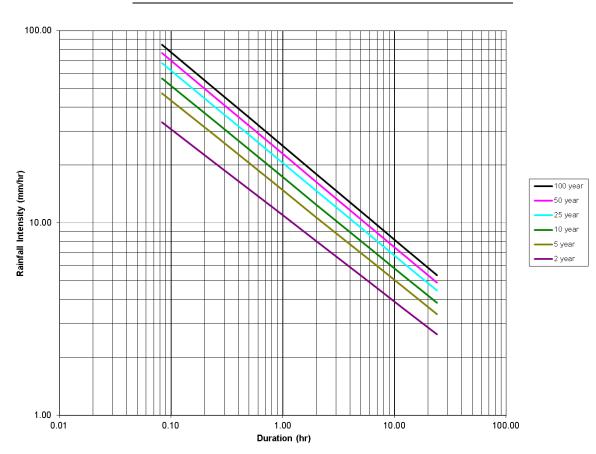


 Table 2. 4701 Piccadilly Place Peak Instantaneous Discharge Estimates



## 4 HYDRAULIC DESIGN

Empirically derived methods (Manning's Equation) were used to size the realigned channel for 100-year design flow. A triangular channel with a 0.6 m top width and 0.3 m depth (at 1H:1V) slopes was found to convey the design flow with 0.2 m freeboard and is similar to the existing channel dimensions downstream of the property. The typical channel section, profile and alignment is shown in the attached



NHC drawing dated March 9<sup>th</sup>, 2015. Rock excavation will be required along the alignment. Details of a cross-section in the rock are provided on the drawing. Alternative channel geometries such as a rectangular channel may be provided if requested.

Stable sediment sizes for channel lining were determined using the method from Aguirre-Pe et al. (2003). The minimum required size of material is based on channel slope. The required size has been defined by median diameter of the stone, with a mean size (based on distribution by mass) ( $D_{50}$ ) ranging from 30 mm gravel to 120 mm cobble (see Drawing, NHC 2015).

Assuming the underlying material is sand and gravel ( $D_5 \ge 15$  mm) or bedrock no filter should be required between the channel lining and the underlying material. However, if the underlying material is substantially finer, a sand and gravel (100 mm thick) or geotextile filter layer is recommended.

The existing channel on the north side of the house should be filled with excavated material and compacted.

# 5 **RIPARIAN AREA**

A previous environmental report pertaining to the watercourse and its potential realignment supported the application for realigning the channel (Ven Huizen Consultants Ltd., 2014). The report found that the watercourse is unlikely to be fish bearing due to the many barriers to fish passage and lack of downstream continuity.

According to the DWV Environmental Development Permit requirements, realignment is expected to result in a no-net loss of riparian habitat (riparian habitat defined by DWV as area within 5 m of the top of bank for non-permanent watercourse). The existing riparian area is limited where the channel is covered by the existing building deck (**Photo 6**) and where the asphalt driveway borders the channel top of banks (**Photo 7**). In these areas no riparian area is considered in the estimate. The existing riparian area is estimated as 394 m<sup>2</sup>.

The proposed structure for development depicted in the appended DNVS design Inc. drawing dated September 2016, follows the 2015 NHC proposed alignment day-lighting the channel to a vegetated area on the eastern boundary of the property. The re-alignment improves the riparian area for approximately 20 m in length. The proposed redevelopment maintains a 1.5 m setback from the right side of the channel to the structure and 5 m riparian offset from the left side of the channel along the length of the property.

Applying a 5 m riparian area to the realigned channel top of banks, limited by any structure or paving encroachment, the proposed riparian area is 498 m<sup>2</sup> which is 104 m<sup>2</sup> greater than existing conditions.



#### **6 CLOSURE**

This document has been prepared by Northwest Hydraulic Consultants Ltd. in accordance with generally accepted engineering practices and is intended for the exclusive use and benefit of Dr. Sukhi Muker and their authorized representatives for specific application to the realignment of the unnamed watercourse on the property at 4701 Piccadilly Place in West Vancouver, BC. The contents of this document are not to be relied upon or used, in whole or in part, by or for the benefit of others without specific written authorization from Northwest Hydraulic Consultants Ltd. No other warranty, expressed or implied, is made.

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We trust that the information provided herein is sufficient for your present needs. Please contact Dale Muir or Nigel Lindsey by email (<u>dmuir@nhcweb.com</u> | <u>nlindsey@nhcweb.com</u>) or by telephone (604.980.6011) if you have any questions or concerns regarding the analyses or recommendations presented herein.

Sincerely,

#### Northwest Hydraulic Consultants Ltd.

Prepared b

Dale Muir, M.Eng, P.Eng Principal

ENCLOSURE



## REFERENCES

- Aguirre-Pe, J., Olivero, m.L., Moncada, A.T. 2003. Particle Densimetric Froude Number for Estimating Sediment Transport. Journal of Hydraulic Engineering. ASCE, pp. 428-437.
- BC MoT (2007). Supplement to TAC Geometric Design Guide. June 2007, 54 pp.
- BGC Engineering Inc. 2009. Metro Vancouver Regional IDF Curves, Metro Vancouver Climate Stations: Phase 1. *Prepared for* Metro Vancouver. December, 2009.
- District of North Vancouver (DNV). 2006. Schedule D.1 Design Criteria Manual. *The Development Servicing Bylaw No. 7388.* December, 2006.
- Ven Huizen Consultants Ltd. 2014. Riparian Assessment, 4701 Piccadilly South, West Vancouver, BC. *Prepared for* Page Guernsey. June 12, 2014.



**PHOTOS** 





Photo 1. Unnamed watercourse originates at culvert outlet on property at Marine Drive. View looking upstream (North) towards Marine Drive.



Photo 2. Manmade drainage channel running west to east along north boundary of property. View looking upstream (west).





Photo 3. Sealed connection at drainage channel to DWV storm water utility.



Photo 4. Road drainage from Marine Drive directed to drainage channel. View looking west on Marine Drive.





Photo 5. Unnamed watercourse in backyard of existing structure, prior to flowing into concrete lined channel under deck. View looking upstream.



Photo 6. Outlet of concrete lined channel under existing deck. View looking upstream towards existing structure.





Photo 7. Manmade pond with concrete weir at wood bridge. Limited riparian area at rock wall/asphalt drive. View looking downstream towards Piccadilly Drive.



Photo 8. Undefined drainage path at rock outcrop near south east corner of property. View looking upstream towards pond outlet.





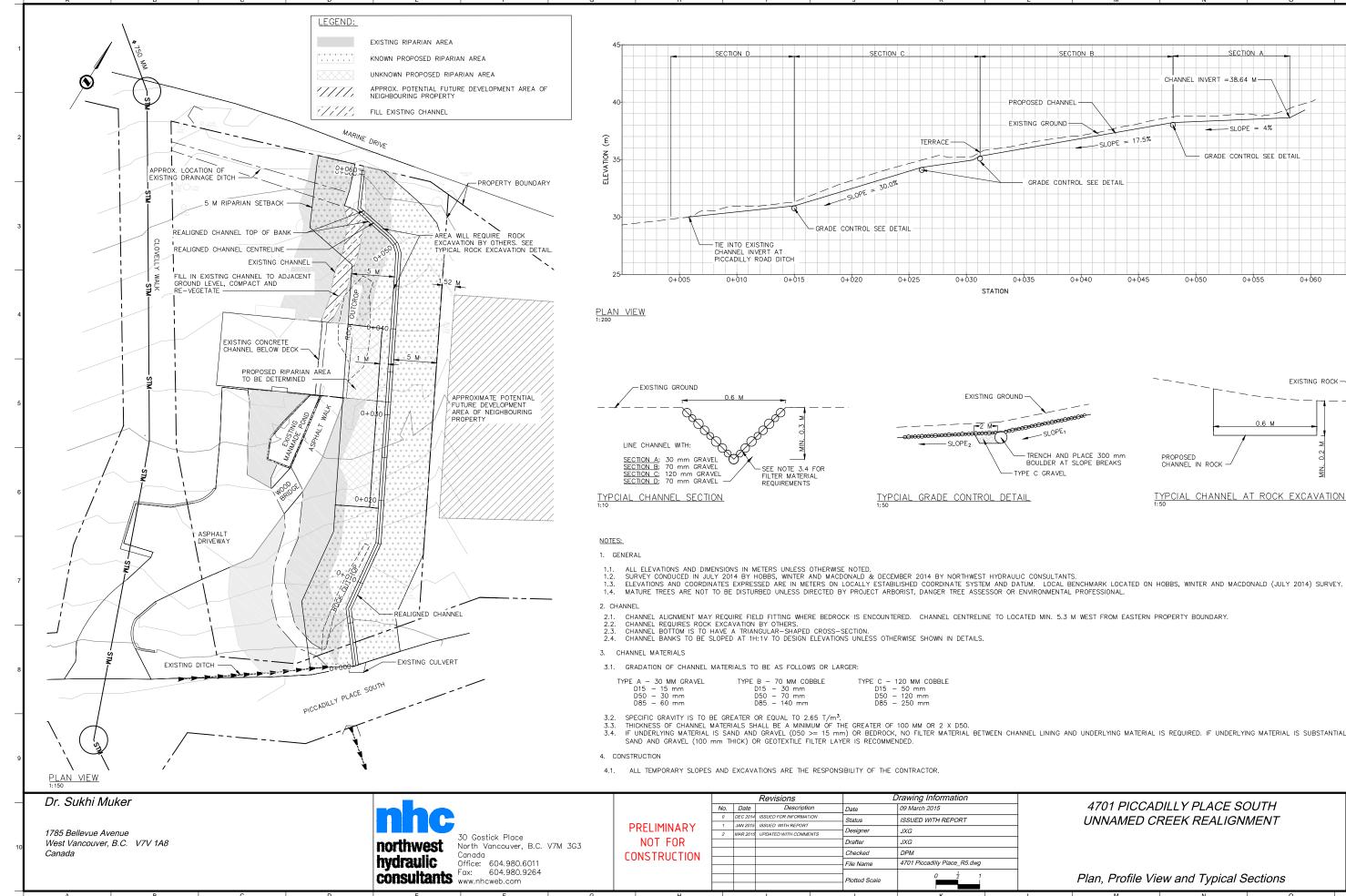
Photo 9. Rock lined channel along 4704 Dogwood Lane. View looking downstream towards Burrard Inlet.



## DRAWING

Attached are:

- initial 2014 December drawing by NHC showing existing site plan and proposed channel alignment
- current 2016 September proposed site layour

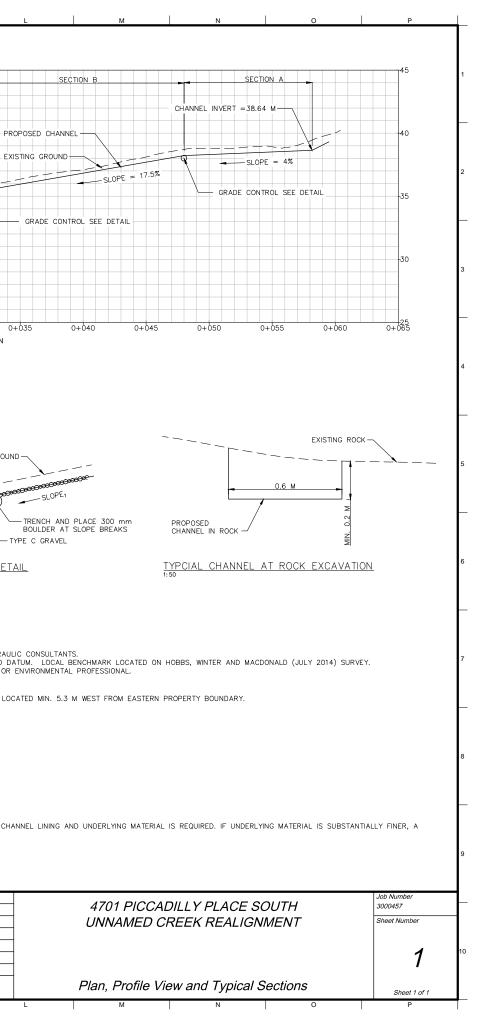


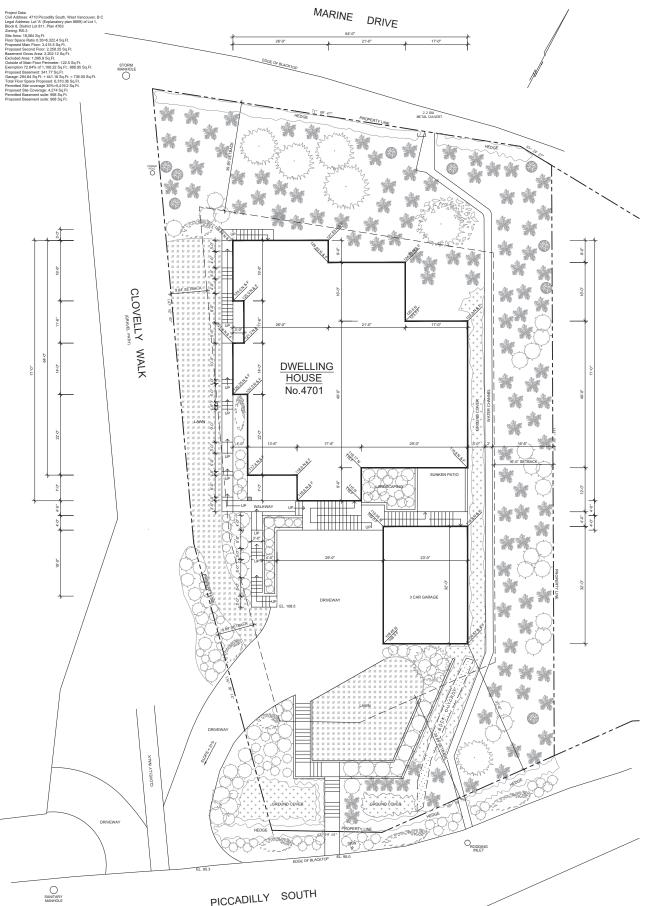
THICKNESS OF CHANNEL MATERIALS SHALL BE A MINIMUM OF THE GREATER OF 100 MM OR 2 X D50. IF UNDERLYING MATERIAL IS SAND AND GRAVEL (D50 >= 15 mm) OR BEDROCK, NO FILTER MATERIAL BETWEEN CHANNEL LINING AND UNDERLYING MATERIAL IS REQUIRED. IF UNDERLYING MATERIAL IS SUBSTANTIALLY FINER, A

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STATION

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PICCADILLY SOUTH

DNVS DESIGN INC.	Project: 4701 Piccadilly South, West	Vancou	iver	Rev	Description of Revisions	Date
Per: Daniela Vidra-Sedler Bachelor in Arch. 250 West 18th Ave. Vancouver BC V5Y 2A7 Phone: 604 266-4629 Fax: 604 264-0322	Sheet Title: Site Plan			3		
	Scale: ½" = 1'-0"		2			
	Date: Sep. 2016	Rev	1		Issued for permit	Sep. 2016
	Drawing No.: A001			$\bigcirc$	Issued for review	May 2016