

## **ENGINEERING MEMO**

Trasolini Chetner Construction Development

FILE NO.: 15-6467

ATTENTION: Rob Chetner

DATE: January 8, 2016

FROM: Joseph (Inseok) Oh, P.Eng.

CC:

**SUBJECT: Stormwater Management Plan** 

LOCATION: 1425 Gordon Avenue, West Vancouver, BC

## **MESSAGE/INSTRUCTIONS:**

As requested, Braun Geotechnical Ltd. (BGL) has prepared a stormwater management plan for the above referenced site.

It is understood that the District of West Vancouver requires a stormwater management system designed to manage surficial stormwater runoff from the subject property. The design criteria are that the post-development peak flow should be restricted to the pre-development peak flow levels for the 10-year design storm event. Rainfall records used for the analysis and design were based on design storm rainfall intensities provided by the Great Vancouver Sewerage and Drainage District for the area - Short Duration Rainfall IDF data for West Vancouver Municipal Hall (VW14).

Table 1 below provides a summary of pre and post development catchment areas types, the equivalent calculated surficial runoff coefficient for the site and peak flows.

Table 1 – Development Stormwater Flow Comparison

	Pre-Cons	truction Cato	hment Area		Post-Const	truction Cate	chment Area
	А	rea	Surface		Ar	ea	Surface
	ft <sup>2</sup>	m²	Coefficient		ft²	m²	Coefficient
Lot	11555.0	1073.5	0.530	Lot	11555.0	1073.5	0.676
House	2383.3	221.4	0.900	Vinson House	2706.5	251.4	0.900
Garage	663.6	61.7	0.900	Laneway Cottage	1399.5	130.0	0.900
Concrete	721.3	67.0	0.900	Garden Cottage	1460.2	135.7	0.900
Pavers	421.0	39.1	0.900	Accessory Buildings	935.7	86.9	0.900
Stone Walkway	359.4	33.4	0.700	Stone Walkway	1120.5	104.1	0.700
Vegetated	7006.4	650.9	0.300	Vegetated	3932.6	365.4	0.300

Pre-development Peak Flows:

Post-development Peak Flows:

O = CIA

O = CIA

 $= 0.530 \times 4.332 \text{ mm/hrs} \times 1073.5 \text{ m}^2 \times 24 \text{ hrs}$ 

 $= 0.676 \times 4.332 \text{ mm/hrs} \times 1073.5 \text{ m}^2 \times 24 \text{ hrs}$ 

 $= 59.15 \text{ m}^3/24 \text{ hrs } (0.68 \text{ l/sec})$ 

 $= 75.49 \text{ m}^3/24 \text{ hrs } (0.87 \text{ l/sec})$ 

Time of Concentration (Pre / Post) = 24 hrs => West Van IDF Curve 10 yr Intensity = 4.332 mm/hr



## **ENGINEERING MEMO**

The site is serviced by an existing ditch located along the south property line. A hydraulic analysis using the rational method showed that for 10-year design storm, the pre-development peak flow is 59.15 m³/24 hrs and the post-development peak flow will be 75.49 m³/24 hrs. Since the post-development peak flow is greater than the pre-development peak flow, stormwater detention will be required to meet the required flow attenuation. The target discharge to the ditch is kept at 59.15 m³/24 hrs. The storage volume is conservatively proposed to be 16.5 m³ without considering infiltration. Any infiltration shall provide as additional factor of safety to storage volume and allows for flexibility with pit subgrade requirements at the time of construction especially considering potential for shallow bedrock. Note that existing site access constraints (south rock wall and north structures) preclude advancement of machine test pits or drill holes at the proposed pit locations.

The proposed detention facility design includes Infiltration/Detention System located at the southwest area of the property. A flow control manhole is proposed at the downstream of the Infiltration/Detention System with release rate equal to pre-development peak flows (59.15 m<sup>3</sup>/24 hrs).

The perimeter drains for Garden Cottage located at the southeast portion of the site should be connected to the pump sump and the ditch in such a manner that prevents stormwater from entering the perimeter drains in the event of a backup, power outage, etc. The pump system including alarm and backup power should be designed by a qualified professional mechanical engineer. BGL should be provided the opportunity to review pump design prior to construction.

Grass / landscaped areas should be underlain by a minimum 0.3m thick layer of permeable topsoil and walkway areas should be sloped to direct flow towards permeable landscaped areas.

Details of the proposed stormwater management system are provided on the enclosed Braun Geotechnical Drawings 15-6467-SWM-1 to -7.

We trust this meets your present requirements.

Encl – Braun Geotechnical Dwg 15-6467-SWM-1 to 7

Braun Geotechnical Ltd.

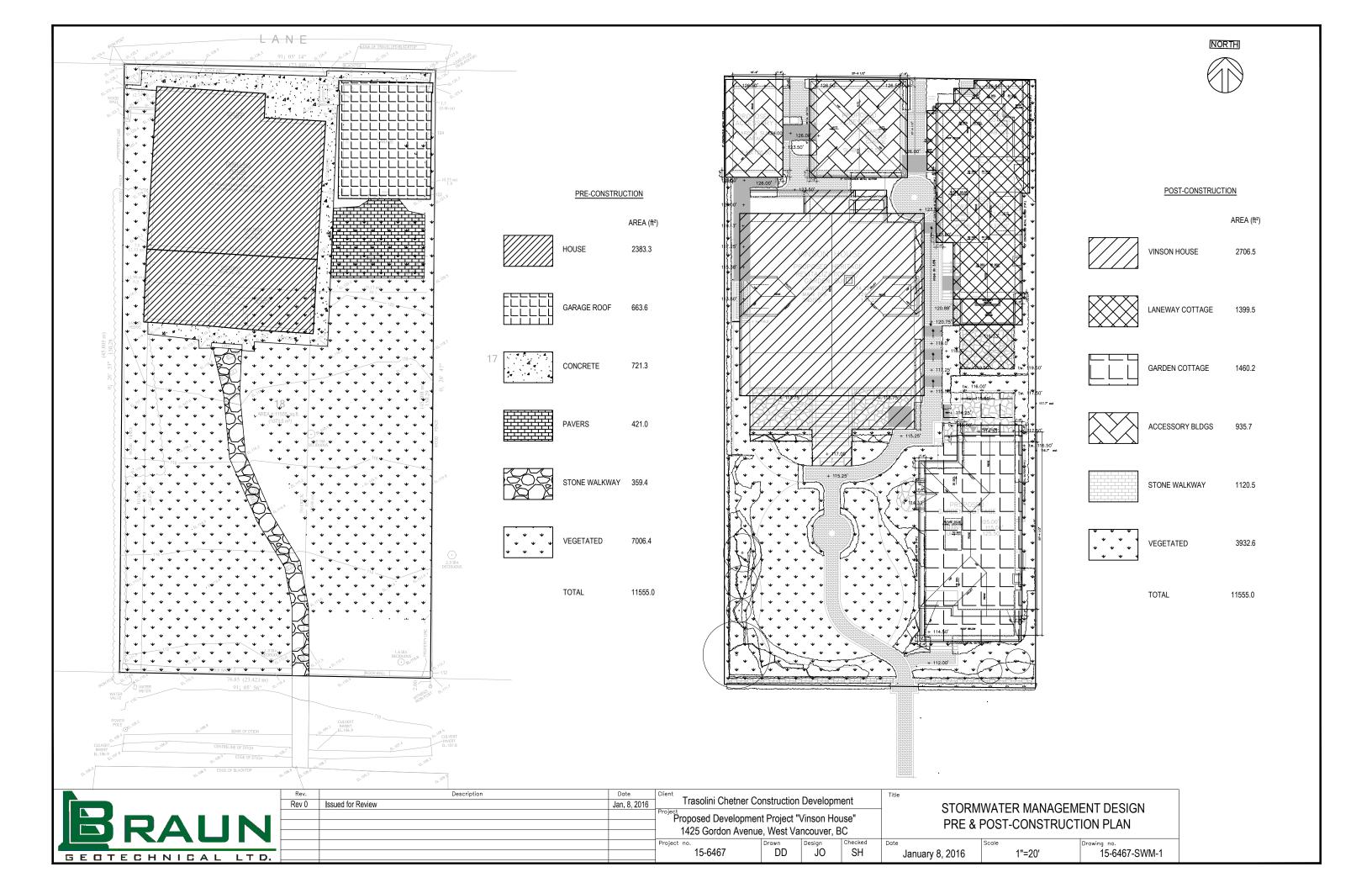
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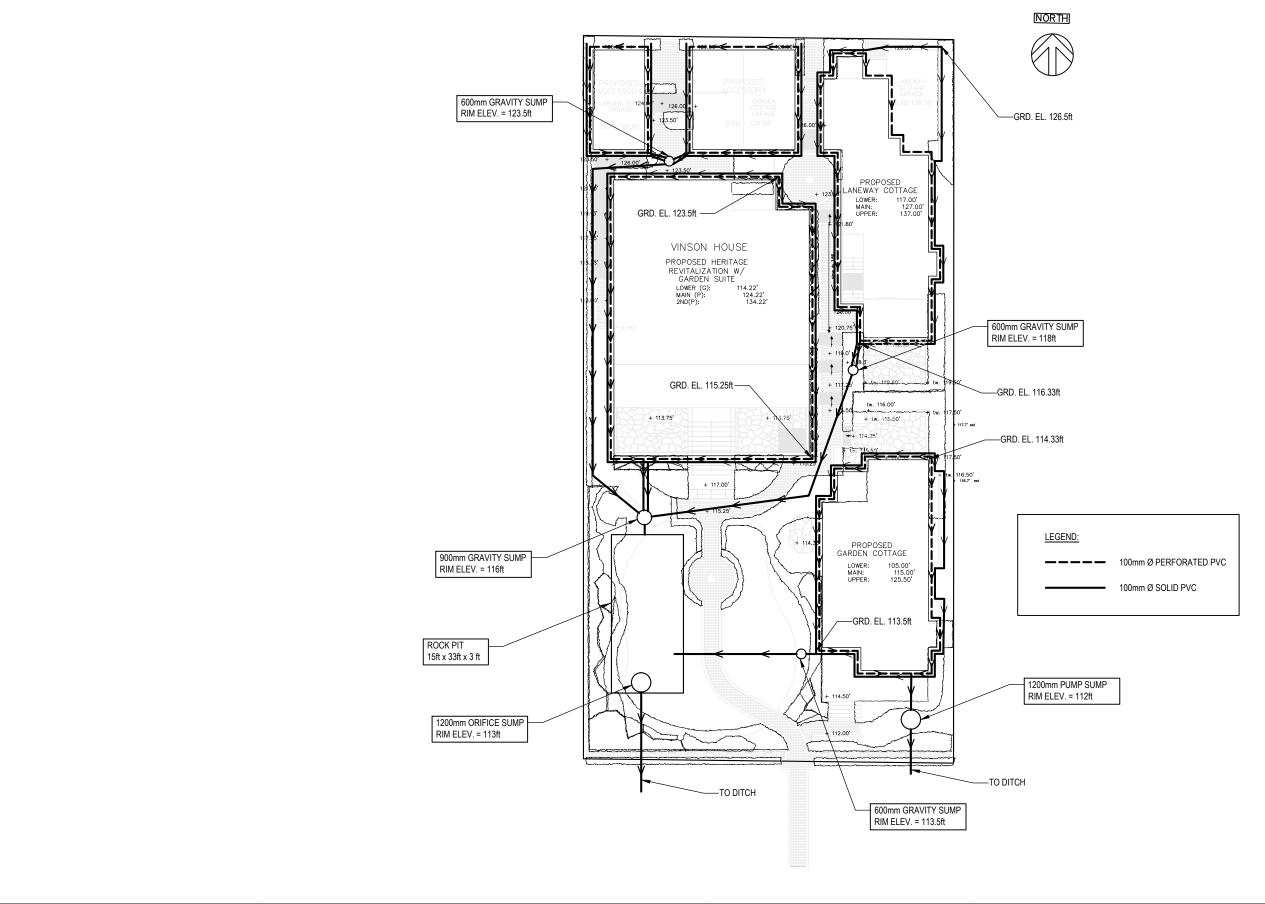
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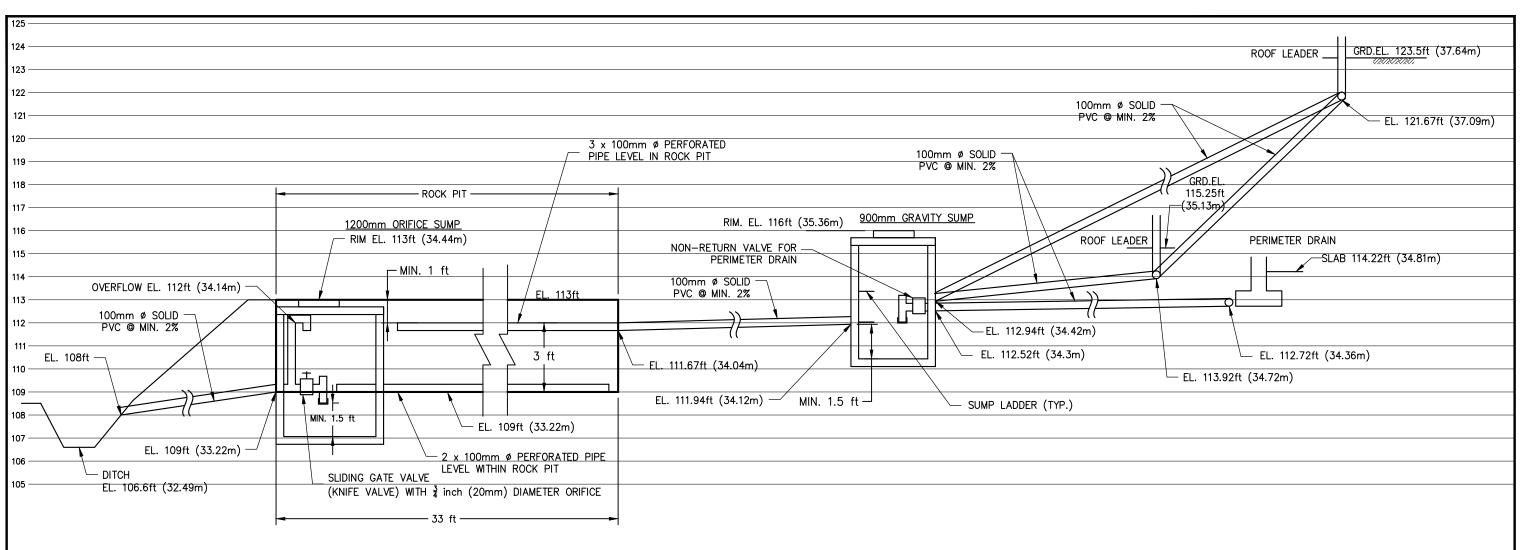




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BRAUN				1425 Gordon Avenue, West Vancouver, BC			
				Project no.	Drawn	Design	Checked
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STORMWATER MANAGEMENT DESIGN	
PLAN	

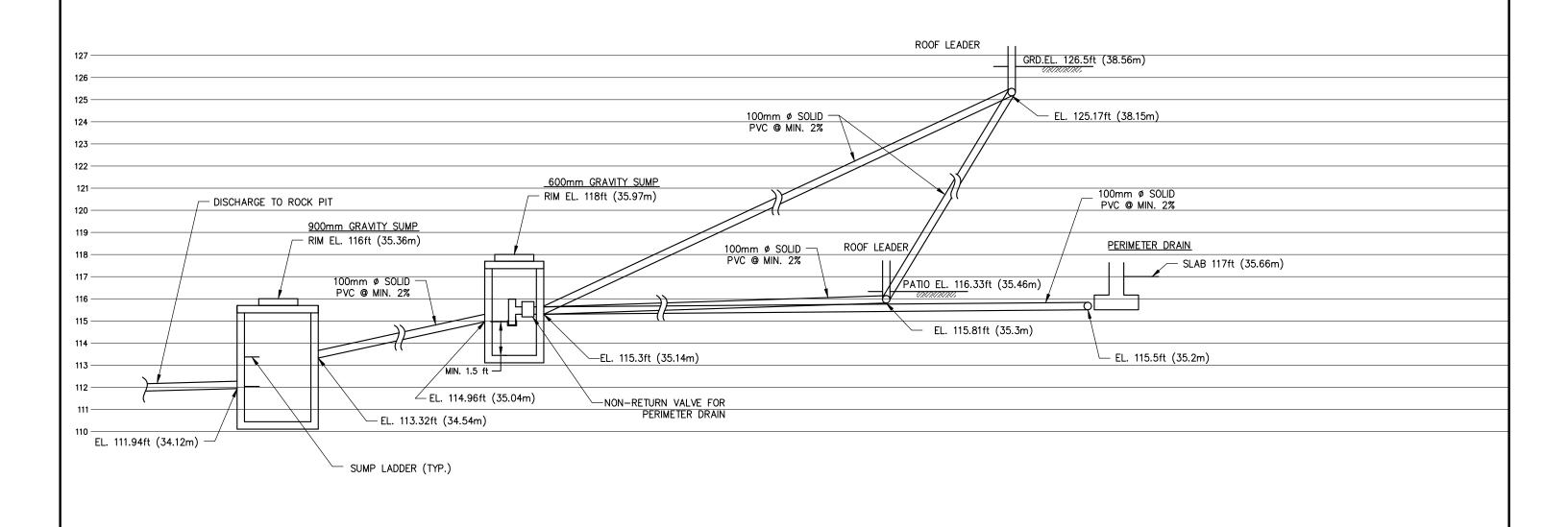
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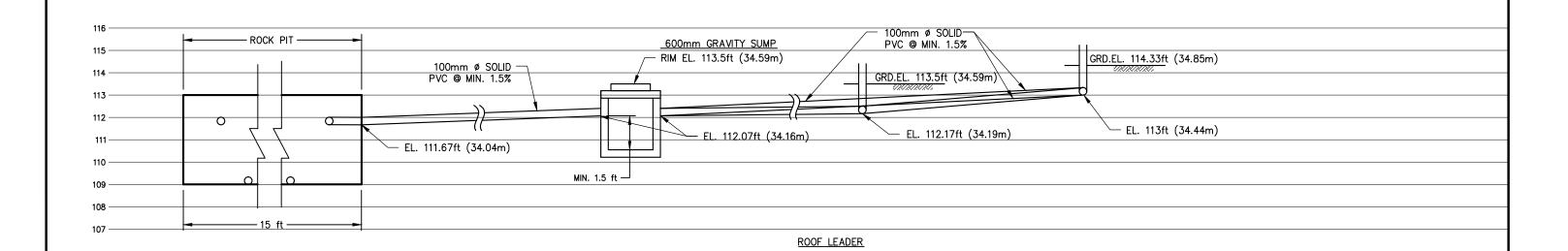
## NOTES:

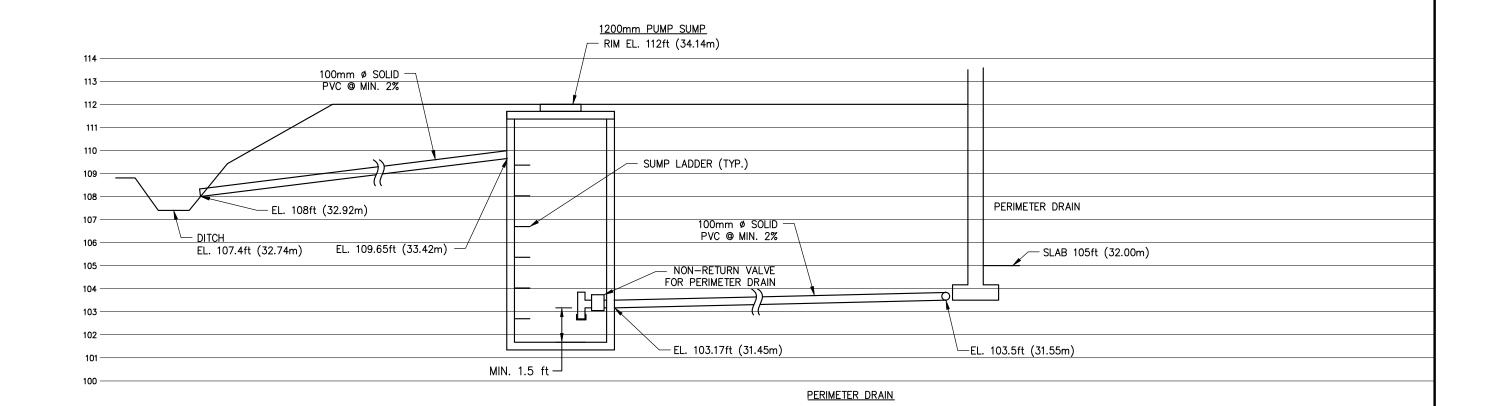
- 1. These drawings are based on Site Plan dated November 2015 by Formwerks Architectural.
- 2. Contractor to confirm all locations and elevations of existing works prior to start of work, and to advise Engineer of discrepancies and conflicts.
- 3. Proposed Storm pipes to be SDR 35 to ASTM D3034 & CAN/CSA-B182.2.
- 4. The Engineer shall be notified at least 24 hrs in advance when pipes and sump are in place and excavation of rock pit is complete, prior to backfill of rock pit and at the time of final lot grading.
- 5. Contractor is responsible for locating all existing underground utilities and connections prior to construction.
- 6. Pump System including alarm and backup power to be designed by a qualified Professional Mechanical Engineer.
- 7. Finished grade around the proposed buildings to be sloped away from the buildings at a minimum slope of 2.0%.
- 8. Pipes to be installed a minimum of 0.3m below finished grade.
- 9. Rock pit should be dimensioned to fit site configuration, ensuring that the minimum required floor area is achieved. The storage volume of the rock pit are based on a void ratio of 40%. Actual dimensions may be revised based on available area, subject to review by Braun Geotechnical.
- 10. Rock pits require periodic maintenance and some care to minimize inlet debris in order to ensure satisfactory performance over the design life. Rock pit repairs and/or total reconstruction can be minimized by regularly cleaning the sump upstream from the pit, and not discharging silty or otherwise contaminated water into the rock pit.

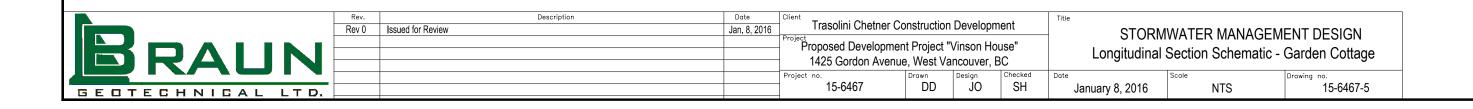
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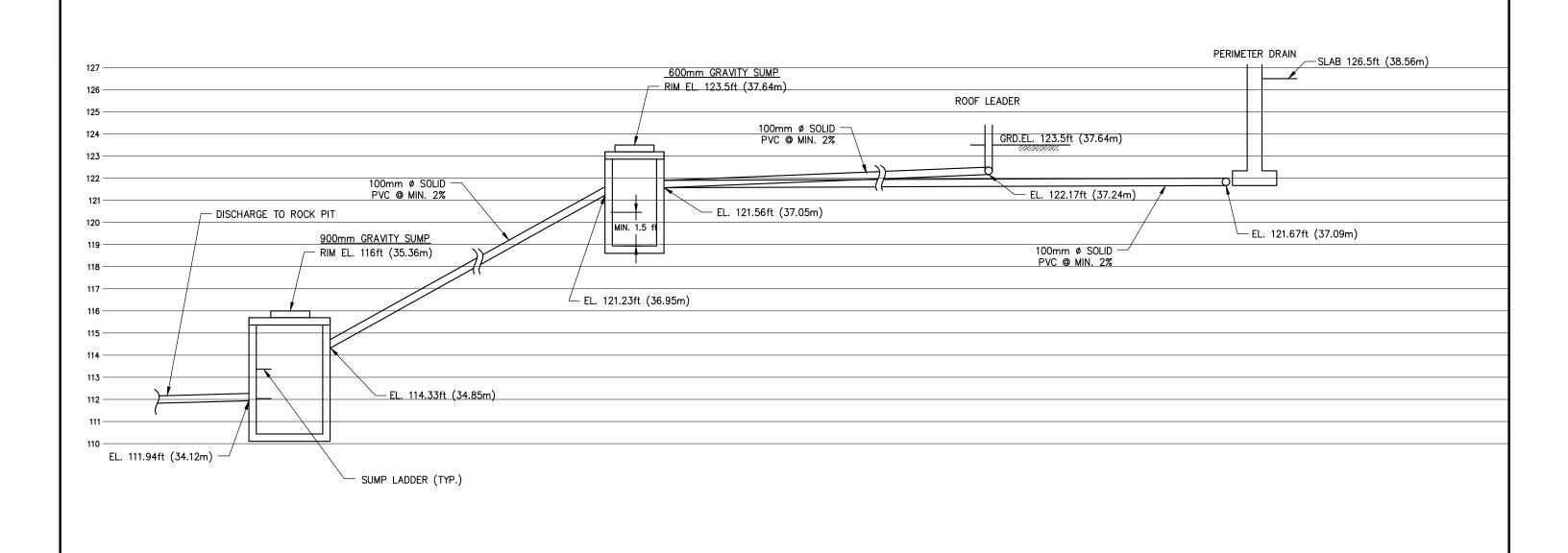


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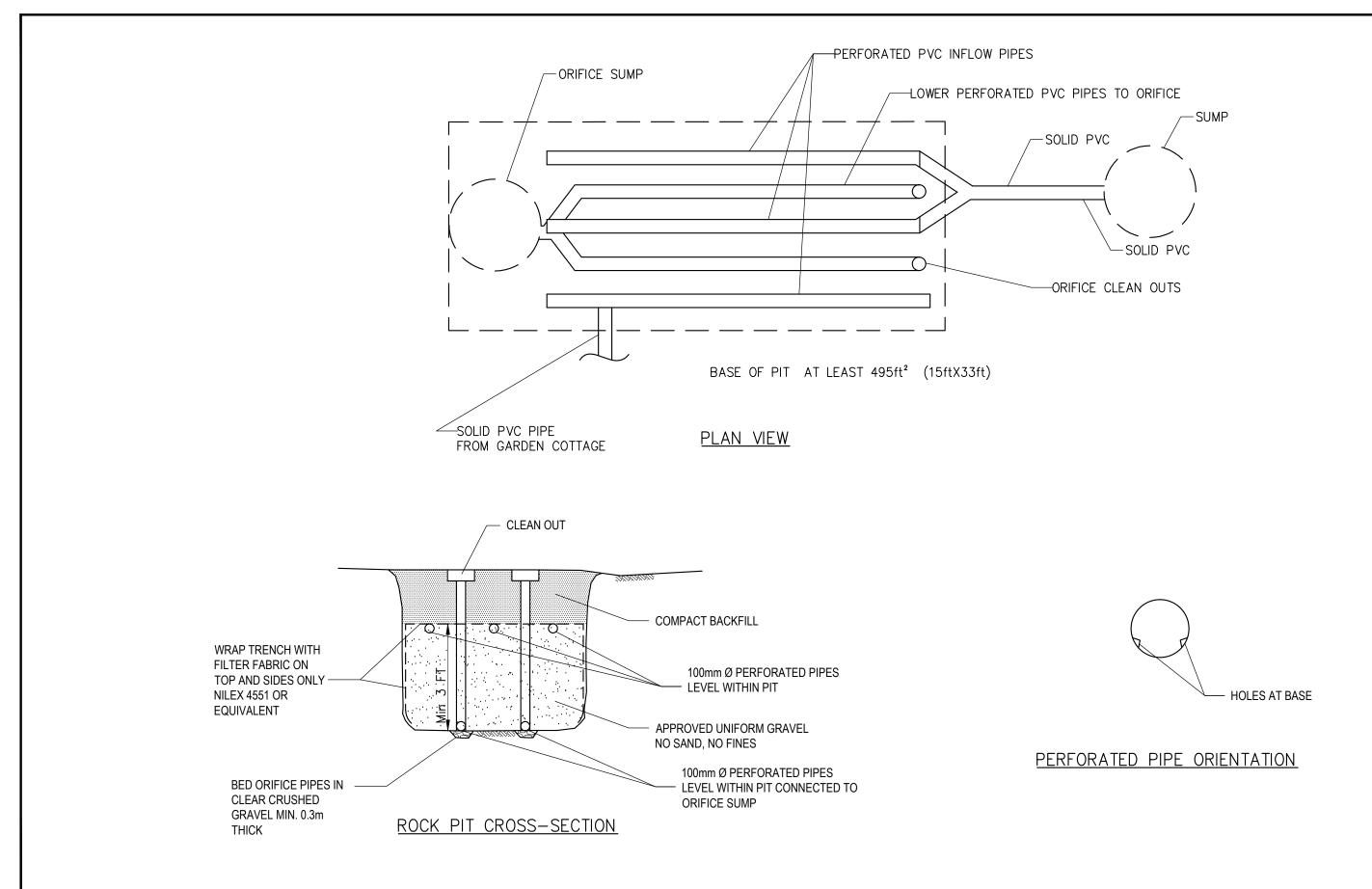








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				Project Proposed Development Project "Vinson House" 1425 Gordon Avenue, West Vancouver, BC			Longitudinal Section Schematic - Accessory Buildings					
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